# COMPARATIVE STRENGTH OF PARTIALLY REPLACED RICE HUSK ASH WITH CEMENT MORTAR A CASE STUDY OF OBUBRA L.G.A OF CROSS RIVER STATE





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## Abstract

The paper assesses and compares the strength of cement mortar that is partially replaced with rice husk ash (RHA). The primary objective of this study is to evaluate the feasibility of using RHA in construction projects as a cost-effective and sustainable alternative to traditional cement-based mortar. The study also seeks to address the pressing environmental issues associated with the disposal of agricultural waste. The study results expect to provide valuable insights into the potential benefits and drawbacks of using RHA in construction projects, particularly in terms of its impact on the durability, strength, and overall quality of the mortar. The idea is to generate wealth by selling RHA to potential buyers instead of disposing of it. Samples of RHA are being mixed into the cement mortar at replacement levels ranging from 5% to 25%. A range of laboratory tests is being conducted to evaluate properties such as particle size distribution, compressive strength, flexural strength, bulk density, and water absorption. The results from the laboratory tests show that as the percentage of RHA increases, the density of the RHA with cement mortar decreases from 2.316 kg/m3 to 1.853 kg/m3. Water absorption of RHA with cement mortar increases from 0.45% to 8.1% as the percentage of RHA increases from 0% to 25%. Compressive strength decreases from 23.4 KN/m2 to 7.31 KN/m2, and flexural strength decreases from 3.93 KN/m2 to 0.54 KN/m2. An optimal percentage of RHA with cement mortar is recommended to be between 10% and 15%.

# Key words: Rice Husk Ash (RHA), Ordinary Portland Cement (OPC), Compressive Strength, Flexural Strength, Water Absorption.

# Introduction

Rice husk is a residue from rice production that accounts for 20% of the 649.7 million tons of rice produced annually worldwide. When incompletely burnt, the husk produced from milling plants contributes to air pollution. An effective approach to tackle the environmental challenge is to explore alternative cementing materials, which can serve as a viable solution. Significant efforts worldwide focus on using indigenous and waste materials such as rice husk as concrete ingredients. If sufficiently ground, the ash produced from controlled burning can be used as a cement replacement material in mortar. Studies have shown that adding rice husk ash (RHA) to Portland cement improves the early strength of mortar and forms a calcium silicate hydrate (CSH) gel around the cement particles. This gel is highly dense and less porous, which may increase the strength of mortar against cracking. The chemical composition of rice husk varies depending on the type of paddy, crop year, and climatic and geographical conditions.(cf:Mehta et al, (1992);Saraswathy and Ha-Won,( 2007)Roy et al (2014)

Rice was introduced into Nigeria in 1920 by Agboola and is believed to have originated from southern India. In Nigeria, rice is cultivated in appreciable quantities in several states, including Ogun, Osun, Niger, Sokoto, Ebonyi, Borno, Benue, and Cross River State. Rice can be cultivated in upland cultivated on normal loamy soil or swamps. Local milling, which involves peeling off freshly covered rice, is mainly done by young men and women using firewood as the heat source. After milling, the husk is obtained, and about 100% of the husk is wasted and could be used as fuel. RHA can enhance the compressive strength of concrete when used as a partial replacement for cement. The optimum replacement level of OPC by RHA to give maximum long-term strength enhancement has been reported between 10% and 30%, with 15% cement replacement by RHA being optimal for achieving maximum strength.(Cf:Mahmud et al. (1996)

However, the use of RHA in the construction industry has yet to be adopted, possibly due to a lack of understanding of its effect on concrete characteristics. Several studies have been conducted on the characteristics of RHA cement mortar in terms of strength and durability. However, only a few researchers have examined the effects of RHA grinding time and its characteristics. (Cf:Akande et al, (2010); Tashima et al, (2004); Zhang et al, (1996). Global rice production is approximately 580 million tonnes annually, increasing as the world's population and rice consumption grow.Pande et al, (2001) Consequently, the paper aims to determine the effect of partially replacing

cement with RHA on the strength of mortar, develop and improve an alternative use of RHA in the construction industries, and provide baseline data for further research on using RHA in cement mortar. By applying supplementary cementitious material, cement can be minimized or reduced while the strength and durability of concrete can be improved compared to conventional concrete. This will help reduce concrete production costs as well as the negative impact on the environment.

# Findings

This explains the processes and methods adopted in collecting data for the research. A descriptive and quantitative research approach was used, and a series of laboratory works experimented to determine the "comparative strength of partially replaced RHA with cement mortar". Cement mortar specimens were collected from the materials laboratory of the Civil Engineering Department, Akwa Ibom State Polytechnic, Ikot Osurua, Ikot Ekpene. The cement used was ordinary Portland cement produced by Lafarge Cement Plc in Calabar, Cross River State. The fine aggregate was mined from a river at Ikot Osurua Community in Ikot Ekpene L.G.A, Akwa Ibom State. RHA was collected from a rice mill site at Obubra L.G.A of Cross River State and oven-dried in the laboratory, while the water supply was from a clean water source in the civil engineering laboratory of Akwa Ibom State Polytechnic. The tests conducted in the laboratory were: (a) particle size distribution, (b) compressive strength, (c) flexural strength, (d) bulk density, and (e) water absorption.

# **Particle Size Distribution Test Result**

The result obtained from the sieve analysis conducted on fine aggregate and RHA is shown in Tables 2.1 and Figures 2.1. The fine aggregate has a uniformity coefficient of 2, implying well-graded sand. The sand fell within zone 3 and is acceptable for making mortar, according to B.S 4551.

Sieve Size	Weight Retained	Percentage Retained	Cumulative Percentage Retained	Percentage Passing
10.0mm	-	-	-	100
6.30mm	10.05	1.01	1.01	98.99
5.0mm	5.50	0.55	1.56	98.44
3.35mm	4.05	0.41	1.97	98.03
2.36mm	16.80	1.68	3.65	96.35
1.18mm	90.05	9.01	12.66	87.34

 Table 2.1: Sieve Analysis of Fine Aggregate.

0.6mm	255.95	25.6	38.26	61.74
425 μ	260.6	26.1	64.36	35.64
300 µ	247.10	24.71	89.07	10.93
150 μ	95.55	9.56	98.63	1.37
75 μ	9.55	0.96	99.59	0.41
Pan	2.55	0.26	99.85	0.15
Summotio	$n \text{ of } M_{\text{esc}} = 1000  a$		<u>.</u>	·

Summation of Mass =1000g

Weight of pan = 116.65g

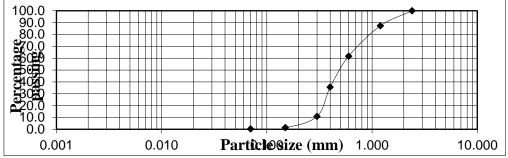


Figure 2.1 Particle size distribution curve for fine aggregate.

## **Compressive Strength Result**

As shown in the result obtained in Figure 2.2, it is seen that the compressive strength of the mortar cubes was below that of the control sample, but between 10-15% replacements, the strength increases above the control strength; this shows that at 10% - 15% RHA replacement, a better compressive strength is achieved. Table 2.2 shows the compressive strength of partially replaced RHA with cement for ages seven days and 14 days for 0% (control), 5%, 10%, 15%, 20% and 25%.

This implies that for a project that requires a higher volume of mortar and 15% of the total volume of the cement required is saved as a result of its partial replacement with RHA, a huge sum of money must have been saved, which can take care of other items in the project, and by so doing reduces the effect of environmental pollution and wastages of the RHA.

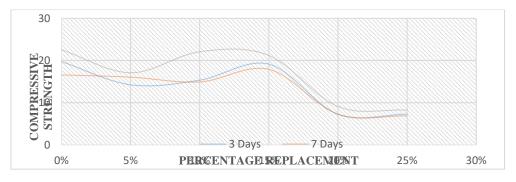


Figure 2.2: Variations of Strength of Mortar and the Percentage Replacement of RHATable 2.2Test Results for Compressive Strength of Mortar at 7 Days

Mark	on	Size	of	Age	Weight	Density	Crushed	Strength
Cubes		Cubes		(Days)	of cubes	$Kg/m^3$	Load	N/mm <sup>2</sup>
		(mm)			(g)	-	(KN)	
C 2		100 x 10	0	7	2284	2.284	154.45	15.45
C 9		100 x 10	0	7	2200	2.200	163.05	16.31
C3		100 x 10	0	7	2180	2.180	180.32	18.03

Table 2.2.2:

#### 5% RHA REPLACEMENT.

			-		-			
Mark	on	Size	of	Age	Weight	Density	Crushed	Strength
Cubes		Cubes		(Days)	of cubes	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
		(mm)		-	(g)	_	(KN)	
No. 1		100 x 10	0	7	2198	2.198	167.18	16.72
No. 2		100 x 10	0	7	2190	2.190	149.13	14.91
No. 5		100 x 10	0	7	2170	2.170	165.66	16.56

Table 2.2.3:

10% RHA REPLACEMENT.

Mark	on	Size	of	Age	Weight of	Density	Crushed	Strength
Cubes		Cubes		(Days)	cubes (g)	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
		(mm)					(KN)	
No. 11		100 x 1	00	7	2246	2.25	192.22	19.22
No. 8		100 x 1	00	7	2236	2.24	176.10	17.61
No. 3		100 x 1	00	7	2178	2.18	79.35	7.94

Table 2.2.4:

#### 15% RHA REPLACEMENT.

Mark	on	Size	of	Age	Weight	Density	Crushed	Strength
Cubes		Cubes		(Days)	of cubes	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
		(mm)			(g)		(KN)	
<b>No. 7</b>		100 x 1	00	7	2154	2.154	131.91	12.43
No. 1		100 x 1	00	7	2126	2.126	183.57	18.35
No. 2		100 x 1	00	7	2152	2.152	229.39	22.94

Table 2.2.5:

#### 20% RHA REPLACEMENT.

Mark on	Size of	Age	Weight	Density	Crushed	Strength
Cubes	Cubes	(Days)	of cubes	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
	(mm)		(g)		(KN)	
No. 1	100 x 100	7	2076	2.076	99.24	9.92
No. 11	100 x 100	7	2064	2.064	57.93	5.79
No. 4	100 x 100	7	2054	2.054	61.74	6.17

Table 2.2.6:

## 25% RHA REPLACEMENT.

Mark on	Size of	Age	Weight	Density	Crushed	Strength
Cubes	Cubes	(Days)	of cubes	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
	(mm)		(g)		(KN)	
No. 10	100 x 100	7	2022	2.022	72.75	7.28
No. 2	100 x 100	7	2128	2.128	69.26	6.93
No. 8	100 x 100	7	2020	2.020	64.10	6.41

#### **Test Results for Compressive Strength of Mortar at 14 Days** Table 2.2.7: **0% RHA REPLACEMENT. (CONTROL)**

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Mark on	Size of	Age	Weight of	Density	Crushed	Strength
Cubes	Cubes	(Days)	cubes (g)	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
	(mm)				(KN)	
C1	100 x 100	14	2358	2.358	216.67	21.67
C4	100 x 100	14	2284	2.284	223.24	22.32
C6	100 x 100	14	2296	2.296	236.08	23.61

## Table 2.2.8: 5% RHA REPLACEMENT.

Mark Cubes	on	Size of Cubes (mm)	Age (Days)	Weight of cubes (g)	Density Kg/m <sup>3</sup>	Crushed Load (KN)	Strength N/mm <sup>2</sup>
No. 12		100 x 100	14	2208	2.21	180.12	18.01
No. 8		100 x 100	14	2200	2.200	150.58	15.06
No. 6		100 x 100	14	2192	2.192	182.17	18.22

#### Table 2.2.9:

## 10% RHA REPLACEMENT.

Mark	on	SIZE of	Age	Weight of	Density	Crushed	Strength
Cubes		Cubes	(Days)	cubes (g)	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
		(mm)				(KN)	
No. 2		100 x 100	14	2276	2.28	228.13	22.81
No. 9		100 x 100	14	2130	2.13	230.40	23.04
No. 11		100 x 100	14	2176	2.18	204.18	20.42

#### Table 2.2.10:

## 15% RHA REPLACEMENT. (CONTROL)

Mark	on	Size	of	Age	Weight of	Density	Crushed	Strength
Cubes		Cubes		(Days)	cubes (g)	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
		(mm)					(KN)	
No. 5		100 x 100	0	14	2126	2.126	230.61	23.06
No. 9		100 x 100	0	14	2168	2.168	212.14	21.21
No. 11		100 x 100	0	14	2168	2.168	193.33	19.33

Table 2.2.11:

#### 20% RHA REPLACEMENT.

Mark	on	Size of	Age	Weight of	Density	Crushed	Strength
Cubes		Cubes	(Days)	cubes (g)	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
		(mm)		_	_	(KN)	
No. 5		100 x 100	14	2072	2.070	83.64	8.36
No. 4		100 x 100	14	2074	2.074	78.85	7.89
No. 8		100 x 100	14	2070	2.070	110.32	11.03

Table 2.2.12:

#### 25% RHA REPLACEMENT.

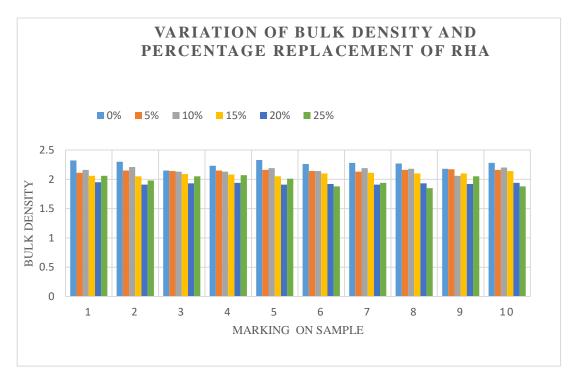
Mark	on	SIZE	of	Age	Weight	Density	Crushed	Strength
Cubes		Cubes		(Days)	of cubes	Kg/m <sup>3</sup>	Load	N/mm <sup>2</sup>
		(mm)			(g)		(KN)	
No. 9		100 x 1	00	14	2180	2.180	104.20	10.42
No. 12		100 x 1	00	14	2098	2.098	74.49	7.45
No. 15		100 x 1	00	14	2188	2.188	68.80	6.88

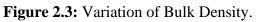
## 2.3 Density Test Results

The results of RHA's effect on cement mortar density are shown in Table 4.3 and Figure 4.3. The density of RHA-mortar decreases as the percentage of the pozzolan increases. This is due to the replacement of Portland cement with a lightweight pozzolan. The specific gravity of cement is greater than that of rice husk ash. The reduction in the bulk density results in lightweight mortar without increasing the pore spaces in the sample.

0	Weight of casted cubes of Cement & RHA mix proportion before curing.									
(Kilogram)           S/No.         0%         5%         10%         15%         20%         25%										
5/1NO.	U%0	5%0	10%	15%	20%	25%				
1	2.316	2.108	2.160	2.064	1.948	2.058				
2	2.264	2.148	2.206	2.046	1.914	1.984				
3	2.152	2.144	2.130	2.094	1.928	2.046				
4	2.234	2.154	2.132	2.080	1.940	2.068				
5	2.330	2.160	2.190	2.048	1.914	2.014				
6	2.258	2.136	2.140	2.096	1.922	1.876				
7	2.278	2.130	2.194	2.108	1.914	1.938				
8	2.272	2.156	2.176	2.098	1.934	1.853				
9	2.180	2.174	2.058	2.100	1.922	2.048				
10	2.282	2.158	2.198	2.138	1.936	1.882				
11	2.250	2.152	2.110	2.084	1.912	2.034				
12	2.276	2.162	2.182	2.106	1.910	1.916				

 Table 2.3: Density of Hardened Mortar (Kg/m<sup>3</sup>)





# 2.4 Results for Water Absorption Test

Table 4.4 and Figure 4.4 show that the highest water absorption was at 25%, followed by 20% RHA replacement. This implies that higher RHA particles possess the highest surface area and fineness and lower reaction ability than cement (Shetty, 2004).

Both of these features result in high water demand to wet the surface area of RHA, as investigated by (Zhang et al., 1996; Ganesan et al., 2008).

According to Habeeb et al., due to the increment of the specific surface area of RHA, more water is required to wet the surface area of RHA. Since then, the water/fine aggregate ratio has been maintained.

Percentage RHA	0	5	10	15	20	25
Average calculated water	0.45	1.345	1.77	1.19	3.6	8.1
absorption (expressed in						
percentage)						

Table 2.4Water Absorption Table

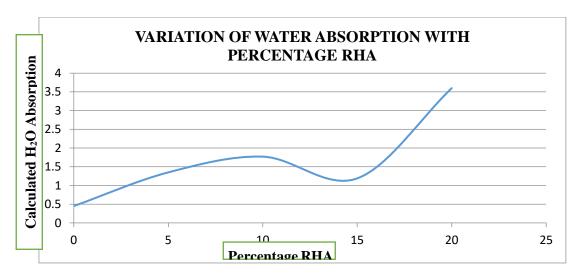


Figure 2.4: Variation of Percentage RHA with Water Absorbed.

# 2.5 Results for Flexural Strength Tests.

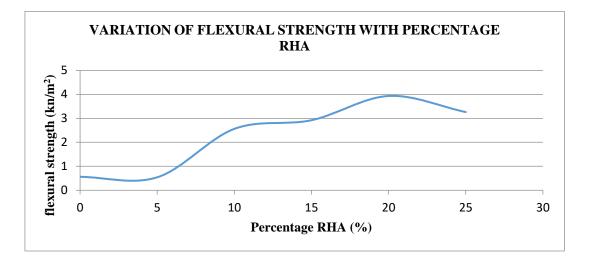
From the result obtained in Table 4.5 and its graphical representation in Figure 4.5, it can be seen that there was a relative increase in flexural strength between (5 -10) per cent RHA replacement.

This is regarded as the safe region from the strength test result because, beyond or below this region, the strength recorded is relatively low compared to the control specimen, which is the (0%) replacement.

Table 2	.5 (7 days strength)		
S/No.	Percentage Replacement	Weight of Cylinder	Load (KN)
		(Kg) + Sample	
1.	25%	10.504	25.7
2.	20%	10.584	24.4
3.	15%	11.102	115.0
4.	10%	11.390	131.4
5.	5%	11.656	176.8
6.	0% (Control)	11.616	146.8

#### **Table 2.5.1**

Percentage RHA (%)	0	5	10	15	20	25
Flexural strength (KN/m <sup>2</sup> )	0.56	0.54	2.56	2.92	3.93	3.26



# **Conclusions and Recommendations Conclusion**

The test results indicate that Rice Husk Ash has a high silica content and can be used partially instead of cement. One potential benefit of this approach could be reducing the exorbitant expenses associated with cement, especially in cases where lightweight concrete and non-loadbearing walls are involved. In addition, agricultural waste such as Rice Husk Ash can benefit construction applications, reducing its negative environmental impact. Rice Husk Ash's compressive strength is high, making it a great option for load-bearing members when used as a 10% to 15% replacement. Finally, it is worth noting that the high surface area of Rice Husk Ash results in a greater water absorption ratio, especially at 20% to 25% cement replacement..

# Recommendations

The findings from the experiment involving the partial substitution of cement with Rice Husk Ash (RHA) to produce construction materials revealed that RHA can be a valuable resource in the construction industry. The study recommends that RHA be primarily used to produce lightweight mortars and non-load-bearing elements such as walls and internal partitions. However, using RHA may also be extended to modify the geotechnical properties of problematic soil during road construction. This modification can be achieved by adding RHA to the soil and mixing it thoroughly to improve its strength, stability, and overall performance. While using RHA in construction has great potential, it must be applied appropriately to ensure its effectiveness and safety.

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